OK

The Granular Monoclinal Wave





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Monoclinal flood wave in water



well described by the Saint-Venant equations, which are two coupled PDEs for



Velocity profile:



Depth-averaged velocity:





The Saint-Venant equations



<u>1. Conservation of mass</u>: (continuity equation)

$$\frac{\partial h}{\partial t} + \frac{\partial \left(h\overline{u}\right)}{\partial x} = 0$$

2. Momentum balance:

$$\frac{\partial}{\partial t}(h\overline{u}) + \frac{\partial}{\partial x}(h\overline{u}^2) = hg \sin \zeta - \frac{\partial}{\partial x}(\frac{1}{2}gh^2 \cos \zeta) + \text{terms involving}$$

$$friction and viscosity$$

In fact, the Saint-Venant equations admit various types of waves:

also known in granular flow

not yet observed in granular flow

also in granular flow Gray & Edwards, JFM **755** (2014)

also in granular flow Boudet *et al.*, JFM **572** (2007)

Now we turn to granular matter



sand avalanches, landslides, etc.

Saint-Venant equations for granular flow

Mass conservation:

$$\partial_t h + \partial_x \left(h \overline{u} \right) = 0$$

Momentum balance:

$$\partial_{t} (h\overline{u}) + \partial_{x} (h\overline{u}^{2}) =$$

$$gh \sin \zeta - \frac{1}{2} \partial_{x} (gh^{2} \cos \zeta) - \mu(h,\overline{u})gh \cos \zeta + v \partial_{x} (h^{3/2} \partial_{x}\overline{u})$$

$$friction with$$

$$friftion with$$

$$friftion with$$

$$friction with$$

$$friction with$$

Special granular features 1: the friction law

$$\partial_{t} (h\overline{u}) + \partial_{x} (h\overline{u}^{2}) =$$
the friction coefficient $\mu(h,\overline{u})$
depends on h and \overline{u}
 $gh \sin \zeta - \frac{1}{2} \partial_{x} (gh^{2} \cos \zeta) - \mu(h,\overline{u})gh \cos \zeta + v \partial_{x} (h^{3/2} \partial_{x}\overline{u})$

And also on the inclination $\boldsymbol{\zeta}$ of the chute:

• In the case of **water**, an arbitrarily small *shear stress* is sufficient to put the fluid in motion.

• Sand, however, remains motionless until the *shear stress* exceeds a certain threshold value => so the slope of the chute (ζ) has to exceed a critical value ζ_1 in order to make the sand flow. On the other hand, the slope must stay below a second critical value ζ_2 because otherwise the sand will rush down in an uncontrollable avalanche. Only in the intermediate interval $\zeta_1 < \zeta < \zeta_2$ we can witness the balance between forces necessary for a steady flow.

Friction coefficient

Pouliquen and Forterre, J. Fluid Mech. 453 (2002)

$$\mu(h,\overline{u}) = \tan \zeta_1 + \frac{\tan \zeta_2 - \tan \zeta_1}{1 + (\beta / L) \frac{h^{3/2}}{\overline{u}} \sqrt{g \cos \zeta}} \quad \text{(for Fr > }\beta\text{)}$$



In our system:

$$\zeta_1 = 32.9^{\circ}$$

 $\zeta = 33.3^{\circ}$
 $\zeta_2 = 42.0^{\circ}$
 $\beta = 0.5$
 $L = 1 \text{ mm}$

Special granular features 2: the viscous term

$$\partial_{t} \left(h\overline{u} \right) + \partial_{x} \left(h\overline{u}^{2} \right) =$$

$$gh \sin \zeta - \frac{1}{2} \partial_{x} \left(gh^{2} \cos \zeta \right) - \mu(h, \overline{u}) gh \cos \zeta + v \partial_{x} \left(h^{3/2} \partial_{x} \overline{u} \right)$$

It plays a key role in preventing the waves from developing infinitely steep fronts.

This effective viscosity arises from the *normal stresses* in the granular sheet.

$$v = v(\zeta) = \frac{2\sqrt{g}}{9} \left(\frac{L}{\beta}\right) \frac{\gamma(\zeta)\sin\zeta}{\sqrt{\cos\zeta}}$$

where $\gamma(\zeta) = \frac{\tan \zeta_2 - \tan \zeta}{\tan \zeta - \tan \zeta_1}$



Basic solution: steady uniform flow

$$\partial_{y}(h\overline{u}) + \partial_{y}(h\overline{u}^{2}) =$$

$$gh\sin\zeta - \mu(h,\overline{u})gh\cos\zeta - \frac{1}{2}\partial_{y}(gh^{2}\cos\zeta) + v\partial_{y}(h^{3/2}\partial_{x}\overline{u})$$

balance between the forces of gravity and friction:

$$u(h,\overline{u}) = \tan(\zeta)$$



ho

$$\tan \zeta_1 + \frac{\tan \zeta_2 - \tan \zeta_1}{1 + (\beta / L) \frac{h^{3/2}}{\overline{u}} \sqrt{g \cos \zeta}} = \tan \zeta$$



We expect monoclinal waves in the range:

Razis, Kanellopoulos & van der Weele, J. Fluid Mech. 843 (2018)



"The proof of the pudding is in the eating" Numerical experiment







J. Fluid Mech. (2018), *vol.* 843, *pp.* 810–846. © Cambridge University Press 2018 doi:10.1017/jfm.2018.149

The granular monoclinal wave

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(Received 11 July 2017; revised 4 December 2017; accepted 1 February 2018)

Balance of forces in the monoclinal wave



Speed of the shock:
$$c = \frac{h_-\overline{u}_- - h_+\overline{u}_+}{h_- - h_+}$$



Travelling wave analysis

We introduce the travelling-wave variable

$$\xi = x - ct$$

and will be concerned with solutions of the form

$$h(x,t) = h(x-ct) = h(\xi)$$
$$\overline{u}(x,t) = \overline{u}(x-ct) = \overline{u}(\xi)$$

Then the mass conservation becomes:

$$-c\frac{dh}{d\xi} + \frac{d}{d\xi}(h\overline{u}) = 0 \quad \text{or:} \quad -ch' + (h\overline{u})' = 0$$

This can be integrated immediately:

$$-ch' + (h\overline{u})' = 0 \implies -ch + h\overline{u} = -K \quad \text{integration} \\ \text{constant} \\ K = h(c - \overline{u}) \\ \hline \text{constant flux} \\ \text{observed in the} \\ \text{co-moving frame} \end{cases}$$

With this ($\overline{u} = c - K h^{-1}$ and hence $\overline{u}' = K h^{-2} h'$, etc.) we can eliminate \overline{u} and its derivatives from the momentum balance:

$$\frac{vK}{h^{3/2}}h'' - \frac{vK}{2h^{5/2}}(h')^2 + \left(\frac{K}{h^3} - g\cos\zeta\right)h' + g\sin\zeta - \mu(h)g\cos\zeta = 0$$

One single second-order ODE for $h(\xi)$!!

Note that with $\overline{u} = c - K h^{-1}$ also the friction coefficient $\mu(h,\overline{u})$ has become a function of **h** alone:

$$\mu(h) = \tan(\zeta_1) + \frac{\tan(\zeta_2) - \tan(\zeta_1)}{1 + \beta L^{-1} \sqrt{g \cos(\zeta)} h^{5/2} / (ch - K)}$$

This ODE (with the proper boundary conditions) gives *all* granular waveforms travelling at constant speed.

In particular also the **monoclinal wave**:



Dimensionless form of the ODE

By measuring all length scales (h and ξ) in terms of the thickness h_{-} of the incoming sheet, and c in terms of the corresponding velocity \bar{u}_{-} , we obtain:

$$\frac{d^2\tilde{h}}{d\tilde{\xi}^2} - \frac{1}{2\tilde{h}}\left(\frac{d\tilde{h}}{d\tilde{\xi}}\right)^2 + \frac{R_{in}\tilde{h}^{3/2}}{F_{in}^2(\tilde{c}-1)} \left[\left(\frac{F_{in}^2(\tilde{c}-1)^2}{\tilde{h}^3} - 1\right)\frac{d\tilde{h}}{d\tilde{\xi}} + \tan\zeta - \mu(\tilde{h})\right] = 0$$

where R_{in} and F_{in} are the Reynolds and Froude number of the incoming sheet:

$$R_{in} = \frac{\overline{u}_{h_{-}^{1/2}}}{\nu(\zeta)} , \quad F_{in} = \frac{\overline{u}_{-}}{\sqrt{h_{-}g\cos\zeta}}$$

$$\tilde{h} = \frac{h}{h_{-}} \qquad \tilde{\xi} = \frac{\xi}{h_{-}}$$

$$\tilde{c} = \frac{c}{\overline{u}_{-}} \propto c h_{-}^{-3/2}$$

Dynamical Systems approach

Our second-order ODE

$$\frac{d^2\tilde{h}}{d\tilde{\xi}^2} - \frac{1}{2\tilde{h}}\left(\frac{d\tilde{h}}{d\tilde{\xi}}\right)^2 + \frac{R_{in}\tilde{h}^{3/2}}{F_{in}^2(\tilde{c}-1)}\left[\left(\frac{F_{in}^2(\tilde{c}-1)^2}{\tilde{h}^3} - 1\right)\frac{d\tilde{h}}{d\tilde{\xi}} + \tan\zeta - \mu(\tilde{h})\right] = 0$$

can also be written as a system of 2 first-order equations:

$$\begin{cases} \frac{d\tilde{h}}{d\tilde{\xi}} = \tilde{s} = f(\tilde{h}, \tilde{s}) \\ \frac{d\tilde{s}}{d\tilde{\xi}} = \frac{\tilde{s}^2}{2\tilde{h}} - \frac{R_{in}\tilde{h}^{3/2}}{F_{in}^2(\tilde{c}-1)} \left[\left(\frac{F_{in}^2(\tilde{c}-1)^2}{\tilde{h}^3} - 1 \right) \tilde{s} + \tan\zeta - \mu(\tilde{h}) \right] \end{cases}$$

Fixed points:

fixed points correspond to flat regions of the flow

$$\frac{dh}{d\tilde{\xi}} = 0 \longrightarrow f(\tilde{h}, \tilde{s}) = \tilde{s} = 0$$
$$\frac{d\tilde{s}}{d\tilde{\xi}} = 0 \longrightarrow g(\tilde{h}, \tilde{s}) = g(\tilde{h}, 0) = 0$$

which translates to $\mu(\tilde{h}) = \tan \zeta$



(the familiar balance between friction and gravity in uniform flow regions), or equivalently:

$$1 - \tilde{c} + \tilde{c}\,\tilde{h} = \tilde{h}^{5/2}$$



Stability of the fixed points

is determined by the eigenvalues of the Jacobian matrix at the fixed points:

$$J = \begin{pmatrix} \frac{\partial f(\tilde{h}, \tilde{s})}{\partial \tilde{h}} & \frac{\partial f(\tilde{h}, \tilde{s})}{\partial \tilde{s}} \\ \frac{\partial g(\tilde{h}, \tilde{s})}{\partial \tilde{h}} & \frac{\partial g(\tilde{h}, \tilde{s})}{\partial \tilde{s}} \end{pmatrix}_{(\tilde{h}_{\pm}, 0)} = \begin{pmatrix} 0 & 1 \\ \frac{\partial g(\tilde{h}, \tilde{s})}{\partial \tilde{h}} & \frac{\partial g(\tilde{h}, \tilde{s})}{\partial \tilde{s}} \end{pmatrix}_{(\tilde{h}_{\pm}, 0)}$$

For the parameter values considered here we find that:

$$(\tilde{h}_+, 0)$$
 is a saddle point
 $(\tilde{h}_-, 0) = (1, 0)$ is an unstable node

Phase portrait





So we have arrived at a **dynamical systems view** of the granular monoclinal wave **in the co-moving frame**, What more can **in dimensionless form**. One ask for!

Concluding remarks

- 1. The elusive granular monoclinal wave has been found.
 - → Thus completing the 1-1 correspondence
 between the "Saint-Venant" waveforms in
 water and sand.

Concluding remarks

- 2. How feasible will it be to detect a granular monoclinal wave in practice?
 - \rightarrow The experimental challenge is:

«How to maximize the ratio $\Delta h/\Delta \xi$ »



Carborundum particles on the Manchester chute:

 β = 0.63, **L** = 0.44 mm, ζ_1 = 31.1° and ζ_2 = 47.5° (Edwards *et al.*, 2017)



Smooth glass beads on the chute in Marseille:



Concluding remarks

- 3. Last but not least, the Dynamical Systems approach can also be used to analyze the other waveforms.
 - \rightarrow Roll waves
 - → Rising waves / granular jumps

Next time, I hope!

Thank you for your attention

- D. Razis, G. Kanellopoulos & K. van der Weele, The granular monoclinal wave, J. Fluid Mech. 843 (2018).
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OK or K.O.?